

A kinetic formulation for a model coupling free surface and pressurised flows in closed pipes

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Non stationnary mixed flows can occurs in closed pipes:

- Free surface flow:

the pipe is not filled by the water which is considered as incompressible

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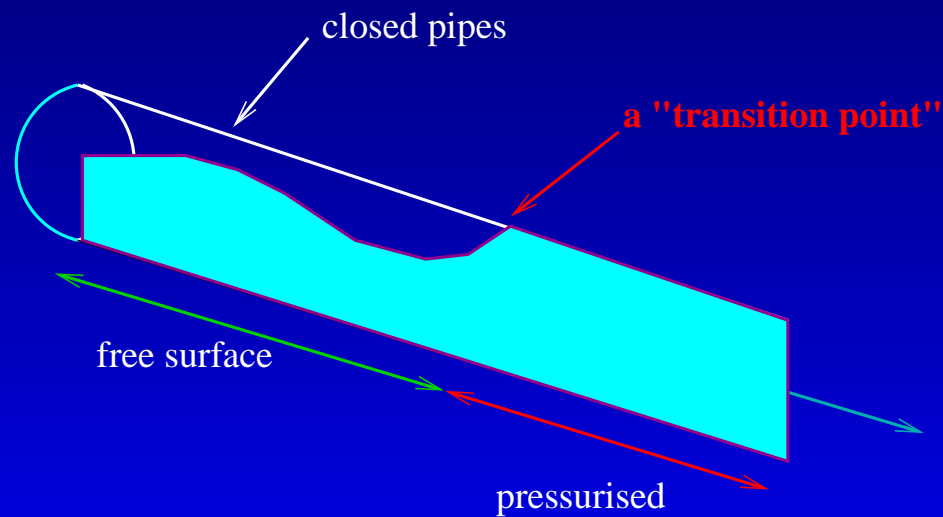
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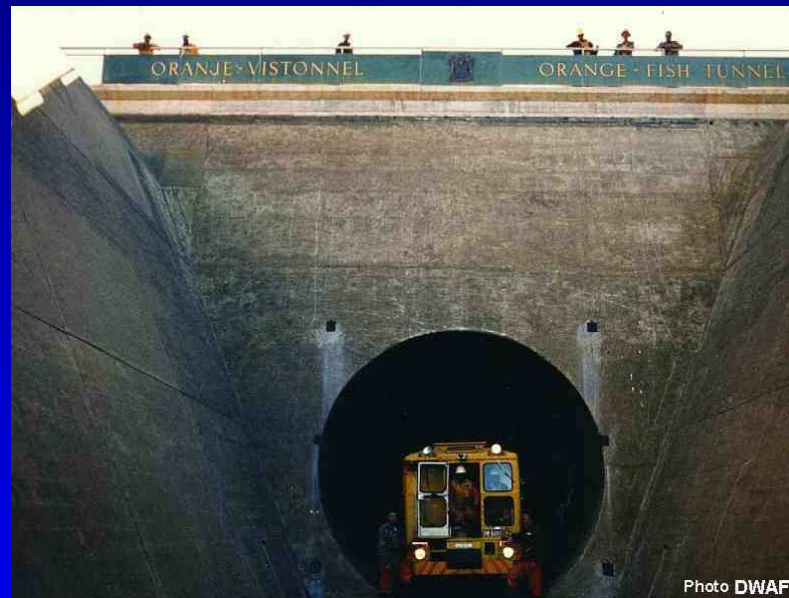
Exemples of closed pipes



a forced pipe



a sewer in Paris



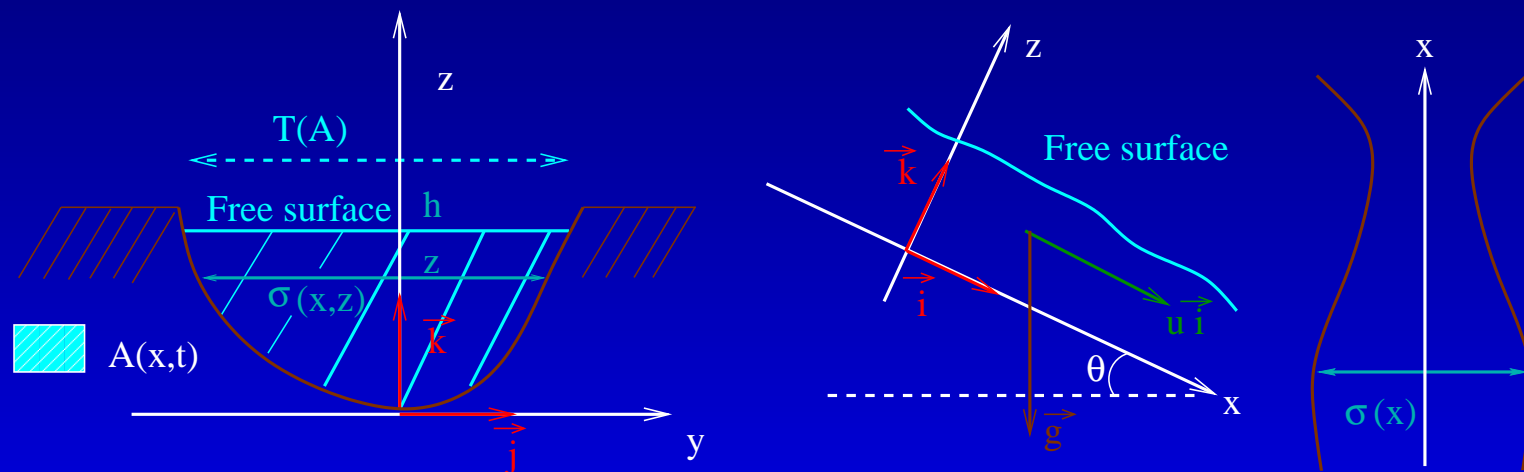
The Orange-Fish Tunnel (in Canada)

Saint Venant system for free surface flows

A the wetted area, Q the discharge.

$$\frac{\partial A}{\partial t} + \frac{\partial Q}{\partial x} = 0$$

$$\frac{\partial Q}{\partial t} + \frac{\partial}{\partial x} \left(\frac{Q^2}{A} + gI_1 \right) = -gA \frac{\partial Z}{\partial x}$$



Under the hydrostatic pressure:

$$I_1 = \int_0^y (y - z) \sigma(x, z) dz$$

Properties of Saint Venant system for free surface flows

- The Saint Venant system is strictly hyperbolic for $A(x, t) > 0$.
It admits a mathematical entropy

$$E(A, Q, Z) = \frac{Au^2}{2} + gAZ + gAh(A) - gI_1(A)$$

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Kinetic formulation of free surface flows

Let $\chi : \mathbb{R} \rightarrow \mathbb{R}$ satisfies:

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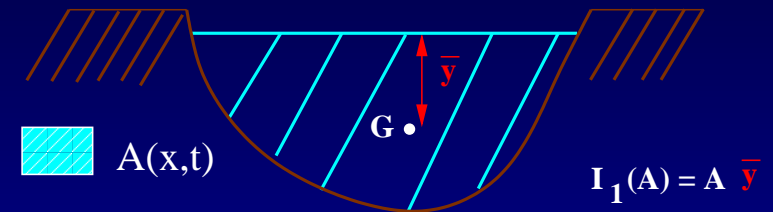
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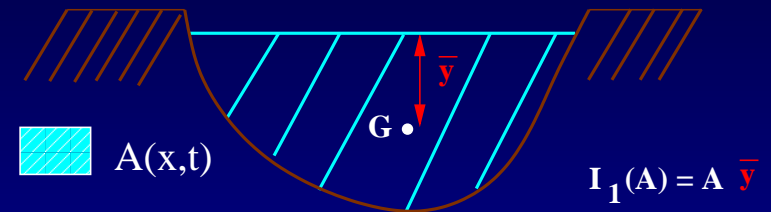
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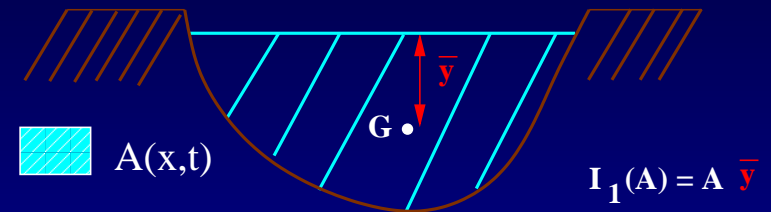
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(A, Q) is a solution of Saint-Venant system if and only if

\mathcal{M} is a solution of the kinetic equation :

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- ▷ **Not convex for the circular case.**

Saint-Venant like system for pressurised flows

3D system of compressible Euler equations + integration over sections orthogonal to the flow axis
+ Boussinesq law for lightly compressible fluid: $P = P_a + \frac{1}{\beta} \left(\frac{\rho}{\rho_0} - 1 \right)$, $c^2 = \frac{1}{\beta \rho_0}$ and
conservative variables: $M = \rho A$, $D = \rho Q$:

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"FS-equivalent wet area" $A_{eq}: M = \rho A_{max} = \rho_0 A_{eq}$

"FS-equivalent discharge" $Q_{eq}: D = \rho Q = \rho_0 Q_{eq}$

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The dual model and its kinetic formulation

$$\begin{aligned}\partial_t A + \partial_x Q &= 0 \\ \partial_t Q + \partial_x \left(\frac{Q^2}{A} + p(x, A, E) \right) &= -g A \partial_x Z\end{aligned}$$

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At each point x of the pipe, and at each time t , if we know the state of the flow (free surface or pressurised), define the Gibbs equilibrium by:

$$\mathcal{M}(t, x, E, \xi) = \begin{cases} \frac{A}{c(A)} \chi \left(\frac{\xi - u(t, x)}{c(A)} \right) & \text{with } c(A) = \sqrt{g\bar{y}(A)} \quad \text{if } E = FS, \\ \frac{A}{c} \chi \left(\frac{\xi - u(t, x)}{c} \right) & \text{with } c = \sqrt{\frac{1}{\beta \rho_0}} \quad \text{if } E = PF \end{cases}$$

Wiggert test

Horizontal rectangular pipe.

Initial condition: free surface still water steady state

Downstream water height increases : a pressurised flows comes from downstream

Downstream water height decreases after

Upstream water level constant until the wave comes ...

